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VOLUME 11 NUMBER 1

Field



Notes

Selected Programs for Hand Calculators in Civil Engineering

Abandoned Strip Mine Reclamation

Washington Office News



FOREST SERVICE

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U.S. DEPARTMENT OF AGRICULTURE

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ENGINEERING FIELD NOTES

Volume 11 Number 1

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FOREST SERVICE
U.S. DEPARTMENT OF AGRICULTURE
Washington, D.C. 20013

EDITOR'S NOTE

This issue introduces some format changes to <u>Field Notes</u>. First, we are now using the Courier type style; this larger, more open type will make this and following issues easier to read. We also are using larger paper; Government agencies now have the option of using the commercial $8 \frac{1}{2} \times 11$ -inch paper size for their publications.

Gordon L. Rome Editor

SELECTED PROGRAMS FOR HAND CALCULATORS

IN CIVIL ENGINEERING

Bernhold Rankenberg Region 5

There are times when it is not practical to use the Regional Computer Center or mini-computers in Civil Engineering, although the recurring nature of some problems (those consisting of a series of arithmetic calculations) makes them amenable to a programmed solution. Use of hand-held programmable calculators, such as those found in most National Forest Engineering Offices, provides the necessary alternative method.

The programs presented here are written for calculators that use algebraic notation, such as Texas Instruments' Model SR-52. Programs usually can be stored on magnetic cards to eliminate the necessity of keying-in the program each time it is used. Without magnetic cards, the keying-in process would be necessary. This not only is tedious for the longer programs, but the keying-in process is -- in itself -- a prime source for introducing errors.

PROGRAM EXAMPLES

Program A: Three-Axle Truck on Simple Beam. This program will calculate maximum moments and shear at any point in a simple beam loaded with a 3-axle truck having axles 14 feet (4.27m) apart. The truck loads are AASHTO HS loads.

The usual procedure is to select a maximum moment from AASHTO's Standard Specifications for Highway Bridges for a given span; then the moment curve is calculated, using the assumption that it is parabolic. That assumption is erroneous, however. The moment for a 40-ft. (12.19m) span, for example, is approximately 6 percent low at the 1/4 point, using the parabolic assumption. The error increases for longer spans; an 80-ft. (24.38m) span has a 20 percent error. This program will not work for off-highway loads or for spans of less than 14 ft. (4.27m).

The moments are based on the shear diagram for the area under the rear axle of the truck. The center moment however, is calculated by placing the center of the span equidistant from the resultant of all axle loads and the nearest axle. It is likely that the area calculation procedure used in this program could be used as well on a moment diagram to calculate deflection by the conjugate beam method.

The shears given produce a shear envelope of maximum and minimum shears for a moving load; they are given until x in Program A is 14 ft. (4.27m) from Ra.

Program B: Moment Distribution. This program will calculate moments in the structure shown. The user must provide fixed-end moments and distribution factors for each joint. The carry-over factor is assumed to be 0.5; therefore, the sections must be uniform. The end joints can be assumed to be completely fixed, or pin-connected, or nonexistent, i.e., a structure with cantilever ends.

This program probably could be adapted to problems in sidesway and temperature expansion. It can be applied to haunched members by going into the program and substituting an appropriate carry-over factor other than 0.5.

Although the operations involved are simple and repetitious, they must be made in a specific sequence and the results must go to a specific location. With eight moments to manipulate, the program is a long one that requires 218 steps.

Program C: Moment of Inertia. This program will calculate the moment of inertia of an "I" section; one or more of the flanges can be eliminated and the program will still work.

This program is especially useful in calculating the moment of inertia of composite sections. To do so, use the given thickness of flange and slab, but calculate an equivalent width for the composite slab and flange using an appropriate N value. The program also is helpful in the conjugate beam method for calculating deflections. As shown in the exhibits, the program produces other data, including the distance of the center of gravity from the bottom flange.

Program D: Mannings Equation. This program solves the Equation 7-36 shown in King's Handbook (5th. Edition), pages 7-13. The solution is by trial and error, but converges rapidly; generally, two or three trials are adequate, depending on the accuracy desired. After the proper depth is calculated to produce a given Q, pressing the RUN button will calculate velocity in fee per second.

This program might be improved by manipulating the equation to derive D as a function of Q, thereby calculating D directly, rather than using a trial-and-error method. However, since X and Y are defined in terms of D, the small increase in convenience does not seem to justify the effort.

EXHIBIT A

PAGE1_OF3
◆ B ×

STEP	PROCEDURE	ENTER		PRESS	DISPLAY
	P ₁ , P ₂ , P ₂	P ₁	STO STO	02	
*	R _A — L — R _B	P ₂	STO	06	
÷) A	Х.	А		Mx
			RCL		$V_{MAX} = R_B = V_X$
	ASSUMPTIONS:	7 L	RCL	05	RA
	1. ∠≥14' (4.27m)		RCL	04	M(MAX)
	2. Ÿ = 18.67' (5.69m)		В		
		127			
		A			
	-				
				-	10 0000 to 10 0000
		L			

LOC	CODE	KEY	COMMENTS	LOC	CODE	KEY	COMMENTS	LOC	CODE	KEY	СОММ	ENTS	LABELS
000	*[LBL				Χ				9			Main Program
		Α				RCL				*			∥ B ™MAX
		ST0		040 152		0				3			C R _B =2P ₂ +P ₁ -R
		0				1				4		_"7	C R _B =2P ₂ +P ₁ -R ₀ D if P ₁ =0
		1				+		080 192)			E
005 117	F	RCL				1				+			A'
		0				4				RCL	1.		В.
		6		045 157)				0			C.
		-)				2			D,
	F	RCL				÷		085 197		X			€'
010 122		0				RCL				(REGISTERS
		1				0				RCL	4		00
		-		050 162		6				0			01 χ
		2				=				6			
		8				STO		090 202		÷			02 P ₁ 03 P ₂
015 127		=				0	RA			2			04 R _A
	I	NV				4				+			05RB(VMAX)
	i f	Pos		055 167		С				1			06 L
		D				RCL				6			07
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020		0				5		201		3	1		09
1		2				Х				3.			10
		Х		060 172		RCL)			11
)				0)	T		12
	R	CL				1		100 212		<i>'</i>	1		13
025 137		0				=				RCL			14
		1				HLT				0			15
		+		065 177		LBL				6			16
		2				В				=			17
		88				RCL		105 217	-		1	1122	18
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- 1.2		+				3				4	A	\neg	FLAGS
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		2		075 187		-		\ \				4	

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-	 	2		075 187		0		1				

EXHIBIT B

TITLE Moment Distribution PAGE 1 OF 4

◆ A ×	◆ B ■	

STEP	PROCEDURE	ENTER		PRESS		DISPLAY
		2.11.211				
	В C	F -				
	7/// 7///					
	ASSUMPTIONS:					
	1. Constant Section.					
	2. JTs "A" & "F" can be					
	any degree of fixity.					
	3. All distr. factors ar	e entered with m	inus si	gns.		
1.		F.E.M CF	ST0	02		
2.		F.E.M. CE	ST0	03		
3.		F.E.M. BD	ST0	04		
4.		F.E.M. FC	ST0	05		
5.		F.E.M. CB	ST0	08		
6.		F.E.M. BC	ST0	09		
7.		F.E.M. BA	ST0	10		
8.		F.E.M. AB	ST0	11		
9.		Distr. factor CE	-	ST-0	12	
10.		Distr. factor CB	-	ST0	13	
11.		Distr. factor BD	-	ST0	14	
12.		Distr. factor BC	-	SŢ0	15	
13.		Distr. factor BA	-	ST0	16	
14.		Distr. factor AB	-	ST0	17	
15.		Distr. factor FC	-	ST0	18	

PAGE	2	OF	4	
PAGE	~	()r	-	

◆ A	K		
		!	

T	4 8		

STEP	PROCEDURE		ENTER			PRESS		DISPLAY
16.	D	istr.	facto	r CF	-	ST0	19	
					A			M @ JT B
					RCL	02		MOM. CF
					RCL	03		MOM. CE
					RCL	04		MOM. BD
					RCL	05		MOM. FC
					RCL	08		MOM. CB
					RCL	09		MOM. BC
					RCL	10		MOM. BA
					RCL	11		MOM. AB
	M _{DB} =1 ₂ MBD if "D" is fixed.							
	MDB = 0 if "D" is pin-							
	connected.							
				-	,			
			-					

	CODE	KEY	COMMENTS	LOC	CODE	KEY	COMMENTS	LOC	CODE	KEY	COMMENTS	LABELS
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		A				0	FEM BD			=	511.00	B ΣM @ JTB
	-	В		040 152		4				ST0		C ZM @ JTC
		Х				RCL				0		D RCL 16
		D	D.F.BA			0		080 192		6		E RCL 15
005 117		÷				6		132		C		A' RCL 13
		2				=				Х		B' RCL 18
		=		045 157		SUM				SBR		C' RCL 14
		SUM		107		1	FEM BA			1'	DF. CF	D RCL 12
		1	FEM AB			0		085 197				E' RCL 17
010 122		1				RCL		197		ST0		\$BR 1' RCL 19
		В				0				0		00 *dsz
		Х		050 162		7				7		01 _
		E	DF BC	102								
		÷				SUM		090				02 FEM CF 03 FFM CF
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		ST0			-+	SUM O	FEM BC	100		CUM		Dr CE
025		0				9	FEM BC	212		SUM 0	FEM CB	01 05
. 137		6									I EN CB	DE RD
		В		065 177		C X				88		15 CF BC 16 DF BA
-		Х		1//						RCL		17 DF AB
			DF BC			SBR 1'	DF CF	105		0		18 DF FC
030		=	D1 BC			<u>.</u> ÷	DF CF	217				19 DF CF
142		ST0										FLAGS
		0		070		2				SUM		0
		7		182		= SUM				2	FEM CF	1
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035		Х Х				<u>0</u> 5	FEM FC	222		RCL		3
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		+	DITIND			0		090 202		*LBL		03
015 127		2				3		202		D'		04
		=				+				RCL		05
		SUM		055 167		RCL				1		06
		1		107		0				2		07
-		0				8		095 207		*RTN		08
020 132		В)		207		*LBL	1	09
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		ST0		060 172		*LBL				RCL		11
		0		172		D				1		12
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		1	-	075 187		RCL						4
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EXHIBIT C

TITLE Moment of Inertia of "I" Section	PAGE_1_OF3
◆ A E	◆ B∝

STEP	PROCEDURE	ENTER		PRESS	DISPLAY
1	Enter height of				
	bottom flange	37	ST0	01	
2	Enter width of bottom flange		ST0	02	
3	Enter height of web		ST0	04	
4	Enter width of web	*	S.TO	05	
5	Enter height of top flange		ST0	07	
6	Enter width of top flange		ST0	08	
7				А	Ad ² + I _o
		V1000	RCL	03	Area Bot. Flg
			RCL	06	Area Web
			RCL	09	Area Top Flg.
			RCL	11.	7 (from Bot.)
			RCL	12	Ad ²
	,				
-2-11-2					

TITLE	Moment	of	Inertia	of	" I "	Section	PAGE	2	OF	3	
PROGRA							DATE				

	CODE	KEY	COMMENTS	LOC	CODE	KEY	COMMENTS	LOC	CODE	KEY	COMM	IENTS	LABELS
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		0		045 157		SUM				RCL			C.
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		1		070		2				2	12	FLAGS
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		*X2				RCL		116		HLT		1
		Χ				0		110 222				2
)35 147		RCL				1						3
		0	_			γ×						4
E		6		075 187		3						

	Manning's Equation for Trape	zoidal Channel			
	PÅK		◆B ≤		
TEP	PROCEDURE	ENTER		PRESS	DISPLAY
	→ e ←				2.000
	7				
	D Z				
	b				
	*				
	This is a trial and error so	olution of the f	ollowin	g equation	on:
			-		
	0 - 1.486(Z+ 1/x) 5/3 D 8/3 S	1/2			
	$Q = \frac{1.486(Z+ \frac{1}{X})}{n[\frac{1}{X}+2(Z^2+1)^{\frac{1}{2}}]^{\frac{2}{3}}} $	_			
	Where Z = e ÷ D				
	X = D ÷ b				
	S = Slope of stream (F	T/FT)			
	N = Mannings "N"	WEI (1997)			
	D = Depth	- 11 CO SA - 45 THE - 11 CO SA -			
	Q = CFS (Known)				-
1.	Enter average side slope as				
,	ratio of horizontal to vert	ical			
	Example: 2:1 = 2	Z	STO	01	
2.	Enter width of bottom	b (Ft)	ST0	02	
3.	Enter Slope	S (Ft)	ST0	0:3	
4.	Enter Manning's "N"	"N"	ST0	04	
5.	Enter Assumed Depth	D (Ft)	ST0	06	THE RESERVE OF THE PERSON OF T
6.				А	Q (CFS)
7.	If "Q" is incorrect, enter	another "D"	ST0	06	

		◆B≭	
PROCEDURE	ENTER	PRESS	DISPLAY
		A	Q
"Q" is correct:		RUN	V (FPS)
	"Q" is correct:	PROCEDURE ENTER "Q" is correct:	PROCEDURE ENTER PRESS A "Q" is correct: RUN

TITLE Moment	Distribution	PAGE 3 OF	4
PROGRAMMER_		DATE	

	CODE	KEY	COMMENTS	LOC	CODE	KEY	COMMENTS	LOC	CODE	KEY	COMMENTS	LABELS
000 112		#LBL				SUM				Α'	+	Main Prog.
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ABANDONED STRIP MINE RECLAMATION

Al Vanderpoel Engineer Wayne-Hosier National Forest

Almost 10,000 acres (4,047 hectares) of the Wayne National Forest in southern Ohio are scarred by abandoned strip mines. In 1978, as a result of a grant from the Appalachian Regional Commission, the Forest Service undertook its first large-scale reclamation of such lands on 40 acres (16.19 hectares) that had been mined soon after World War II.

Known as the Yost Tract, this area was characterized by 50-ft. (15.24m) vertical walls, strip pits filled with acid water, and mounds of overburden material (Fig. 1). Because of the exposed coal residues, there was almost no vegetation, and surface water runoff was very acidic; the pH level in the soils ranged from less than 3.5 to 5.5.

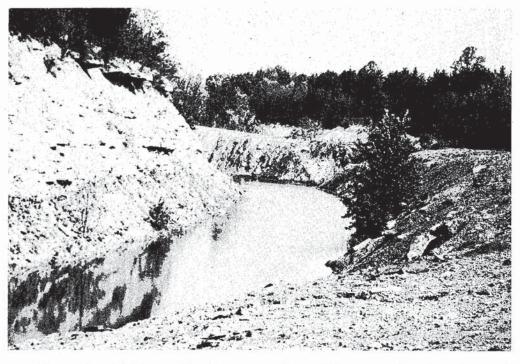


Figure 1. -- Conditions before reclamation work showing highwall, strip pit, and spoil bank

A Forest Service team of specialists developed guidelines that were used in preparing a contract package to reclaim the site, and in developing a Land Use Plan for the area. The plan consisted of:

- 1. Draining the acid water ponds;
- 2. Burying as much acid material as possible, and covering the site with a minimum of 24 inches (60.96cm) of nontoxic material;
- Restoring the land to its original contour (to the top of the vertical walls);
- 4. Covering the area with 12 inches (30.48cm) of topsoil;
- Cleaning and opening drainage-ways;
- 6. Liming, fertilizing, and revegetating the site with grasses immediately; and
- 7. Planting a variety of tree species that seemed appropriate to the site.

A public works contract was awarded for this project in April 1978; the earthwork was subcontracted to a coal mining firm experienced with this type of reclamation work. Using D-9 dozers for clearing, grubbing, and earthwork, the area was leveled initially and the acid ponds drained, after which the site was shaped to final grade (Fig. 2). Scrapers were used to haul borrow material for covering 10 (4.047 hectares) of the 40 acres (16.19 hectares) with 12 inches (30.48cm) of cover soil; the 10 acres chosen were the worst soils on the site.

New waterways were constructed to replace the existing drainages on the site, using stone riprap (Fig. 3). Also, diversion ditches were cut halfway down the finished slope to intercept runoff. The contractor's work was completed when the entire area was lined heavily and then fertilized, seeded, and covered with hay as mulch; an antierosion disk was used to crimp the hay into the soil. Figure 4, the completed project, is the same location as shown in Figure 1.

Three D-9 dozers, two scrapers, and a front-end loader were used in the project, which was completed in 122 days at a cost of \$4,950 per acre (.4047 hectare). The time and cost compare favorably with the State and Soil Conservation Service projects in the area. Downtime was essentially zero, but the work was delayed 4 weeks because of wet weather.

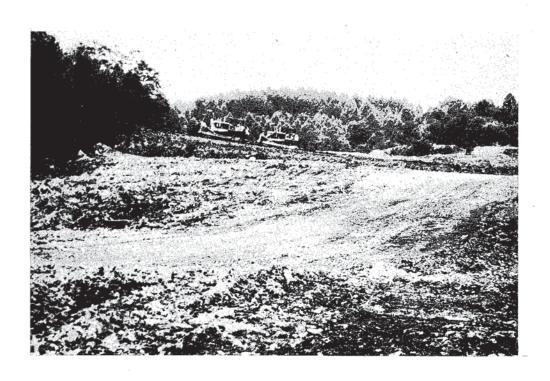


Figure 2. -- Dozers push material to final grade



Figure 3. -- New waterways established on site



Figure 4. -- Completed project at same location as Figure 1

Based on this work, some recommendations that could be applied to similar projects in the future are:

- Reduce construction and overhead costs by using a service contract. Calculate earthwork quantities from photogrammetry, rather than by field work, or by use of design quantities.
- Cut the amount of earthwork required by restoring the land to other than its original contour.
- Accommodate the difficulty in sorting excavation material to obtain topsoil in the planning stage; thus, borrow material is needed.
- 4. Note, in planning, that the material excavated in this particular type of site may expand about 30 percent.

WASHINGTON OFFICE NEWS

CONSULTATION & STANDARDS

Walter E. Furen Assistant Director

COST-EFFECTIVENESS ANALYSIS IN ENGINEERING MANAGEMENT

Originally developed by civilian economists in 1936, "benefit-cost analysis" evolved naturally into "cost-effectiveness analysis" -- a familiar tool for economic evaluation of complex national defense and space exploration systems. That tool can be applied efficiently to routine engineering management problems, particularly for situations in which personnel, equipment, or dollar resources are restricted.

Simple cost-effectiveness analysis of a functional specialty (such as geotechnical engineering) can be used:

- to obtain maximum benefits available from a limited program effort;
- to determine investigation intensity levels required for the program;
- to provide assurance that work levels are commensurate with project opportunities, risks, and values; and
- to develop an awareness of operating costs, as well as unit costs, for meaningful work segments.

SITUATION EXAMPLES

Suppose that laboratory work is being accomplished by force account methods, and that commercial laboratory facilities are available to do that work under contract. The cost elements in this situation then can be compared directly on a unit cost basis.

To provide a comparison base, determine the relative importance of other pertinent elements, such as responsiveness (turnaround time), accuracy, availability of equipment, and other factors. Then,

independently evaluate each element (including cost) for each alternative, and compute a weighted rating for those factors. The weighted ratings are a means for comparing the alternatives.

Determine Priorities for Projects and Programs.

- For each unit of work, compute the "fixed cost" to be expended for the estimated time to be dedicated to the work, including the cost of personnel, equipment, and supplies.
- 2. Identify the project/program objectives and make a comparative rating for each. Evaluate the predicted accomplishments in terms of each objective.
- 3. The weighted ratings computed for each project/program can be used to determine the greatest potential benefit to be derived in return for expenditures of limited resources (personnel, equipment, supplies, and dollar equivalents).

Although the techniques for cost-effectiveness analysis were developed to solve highly-complex problems in economics, the methods -- coupled with rational decision analysis -- become extremely meaningful tools for an engineering manager in making decisions on a day-to-day basis.

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TECHNOLOGICAL IMPROVEMENTS

Heyward T. Taylor Assistant Director

SOLAR POWERED VAULT TOILET VENTING SYSTEM

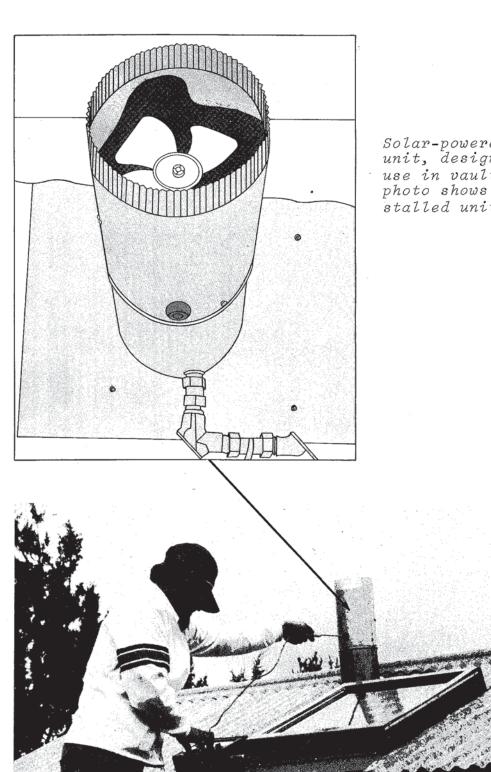
There are some 27,000 vault toilets located in the National Forests. The natural odors associated with those installations are disagreeable to the people using and maintaining those structures; consequently, there is a tendency to avoid using them, as well as to postpone or avoid servicing the units.

Other problems are related to the disagreeable conditions; the incidence of vandalism is greater, and people use the area around the building instead of the facilities provided. Increased vandalism results in increased maintenance costs, and the human wastes on the ground can become health hazards that are transmitted by insects, rodents, and water.

The San Dimas Equipment Development Center (SDEDC) has been testing systems to provide ventilation and to reduce the objectionable odors in the buildings. Several general patterns and constraints were identified during the preliminary work:

- The buildings are used more frequently during daytime than at night;
- 2. They are used more frequently during clear weather than during inclement conditions;
- Because the units are isolated, access to power sources is severely limited;
- Servicing of ventilating equipment must be simple; and
- 5. Costs for installation must be low.

The SDEDC prototype for the ventilation system consisted of a 20-watt solar array supplying electrical energy to a small fan motor in a 6-inch (15.2cm) diameter stack for the roof of the building (See Figure). The installation requires only small modification to existing structures.



Solar-powered ventilating unit, designed by SDEDC for use in vault toilets. The photo shows a completely installed unit.

The units were tested in Regions 4 and 5, and they proved satisfactory. The solar array provides sufficient energy on clear, sunny days to aspirate the structures at a maximum of .750 cf/minute (229cm/minute); this results in an exchange of all the air in the vault every few minutes. Since the most frequent use occurs during the daytime, no backup electrical system was required, and the usual regulator, batteries, and diodes were unneccessary.

The Department of Energy has funds available for other Federal agencies to purchase solar arrays for use in the development of solar energy systems. The Forest Service plans to install about 500 SDEDC ventilation systems at selected sites throughout the United States (about 2 percent of the existing vault toilet installations). The cost will run about \$235 per unit.

The results of this project will be shared with other Federal, State, and local agencies.

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